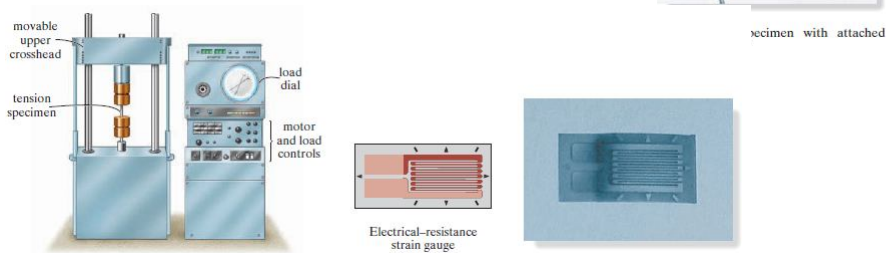


## Mechanical Properties of Materials

### 3.1 The Tension and Compression Test

The strength of a material depends on its ability to sustain a load without undue deformation or failure. This property is inherent in the material itself and must be determined by *experiment*. One of the most important tests to perform in this regard is the *tension or compression test*. Although several important mechanical properties of a material can be determined from this test, it is used primarily to determine the relationship between the average normal stress and average normal strain in many engineering materials such as metals, ceramics, polymers, and composites.



1

### 3.2 The Stress–Strain Diagram

It is not feasible to prepare a test specimen to match the size,  $A_0$  and  $L_0$ , of each structural member. Rather, the test results must be reported so they apply to a member of *any size*. To achieve this, the load and corresponding deformation data are used to calculate various values of the stress and corresponding strain in the specimen. A plot of the results produces a curve called the *stress–strain diagram*. There are two ways in which it is normally described.

**Conventional Stress–Strain Diagram.** We can determine the *nominal* or *engineering stress* by dividing the applied load  $P$  by the specimen's *original* cross-sectional area  $A_0$ . This calculation assumes that the stress is *constant* over the cross section and throughout the gauge length. We have

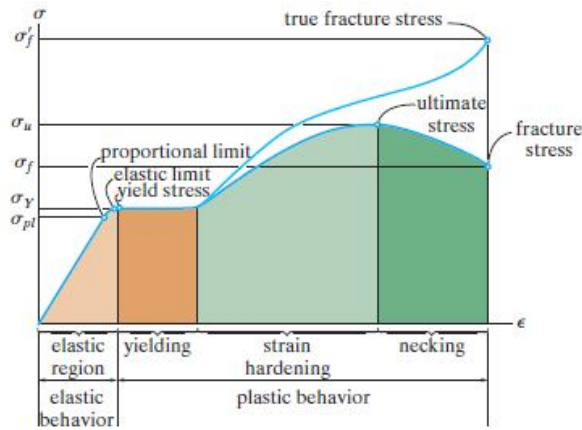
$$\sigma = \frac{P}{A_0} \quad (3-1)$$

Likewise, the *nominal* or *engineering strain* is found directly from the strain gauge reading, or by dividing the change in the specimen's gauge length,  $\delta$ , by the specimen's original gauge length  $L_0$ . Here the strain is assumed to be constant throughout the region between the gauge points. Thus,

$$\epsilon = \frac{\delta}{L_0} \quad (3-2)$$

2

We will now discuss the characteristics of the conventional stress-strain curve as it pertains to *steel*, a commonly used material for fabricating both structural members and mechanical elements. Using the method described above, the characteristic stress-strain diagram for a steel specimen is shown in Fig. 3-4. From this curve we can identify four different ways in which the material behaves, depending on the amount of strain induced in the material.



Conventional and true stress-strain diagrams for ductile material (steel) (not to scale)

3

### 3.3 Stress-Strain Behavior of Ductile and Brittle Materials

Materials can be classified as either being ductile or brittle, depending on their stress-strain characteristics.

**Ductile Materials.** Any material that can be subjected to large strains before it fractures is called a *ductile material*. Mild steel, as discussed previously, is a typical example. Engineers often choose ductile materials for design because these materials are capable of absorbing shock or energy, and if they become overloaded, they will usually exhibit large deformation before failing.

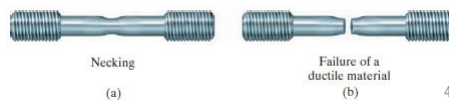
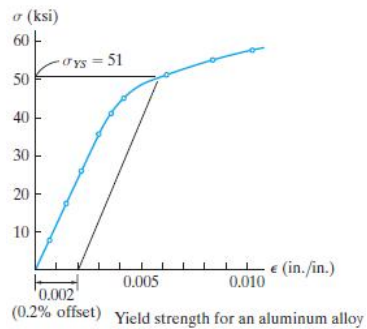
One way to specify the ductility of a material is to report its percent elongation or percent reduction in area at the time of fracture. The *percent elongation* is the specimen's fracture strain expressed as a percent. Thus, if the specimen's original gauge length is  $L_0$  and its length at fracture is  $L_f$ , then

$$\text{Percent elongation} = \frac{L_f - L_0}{L_0} (100\%) \quad (3-3)$$

As seen in Fig. 3-6, since  $\epsilon_f = 0.380$ , this value would be 38% for a mild steel specimen.

The *percent reduction in area* is another way to specify ductility. It is defined within the region of necking as follows:

$$\text{Percent reduction of area} = \frac{A_0 - A_f}{A_0} (100\%) \quad (3-4)$$



4

### 3.4 Hooke's Law

As noted in the previous section, the stress–strain diagrams for most engineering materials exhibit a *linear relationship* between stress and strain within the elastic region. Consequently, an increase in stress causes a proportionate increase in strain. This fact was discovered by Robert Hooke in 1676 using springs and is known as *Hooke's law*. It may be expressed mathematically as

$$\sigma = E\epsilon \quad (3-5)$$

Here  $E$  represents the constant of proportionality, which is called the *modulus of elasticity* or *Young's modulus*, named after Thomas Young, who published an account of it in 1807.

Equation 3-5 actually represents the equation of the *initial straight-lined portion* of the stress–strain diagram up to the proportional limit. Furthermore, the modulus of elasticity represents the *slope* of this line. Since strain is dimensionless, from Eq. 3-5,  $E$  will have the same units as stress, such as psi, ksi, or pascals. As an example of its calculation, consider the stress–strain diagram for steel shown in Fig. 3-6. Here  $\sigma_{pl} = 35$  ksi and  $\epsilon_{pl} = 0.0012$  in./in., so that

$$E = \frac{\sigma_{pl}}{\epsilon_{pl}} = \frac{35 \text{ ksi}}{0.0012 \text{ in./in.}} = 29(10^3) \text{ ksi}$$

5

### Important Points

- A *conventional stress–strain diagram* is important in engineering since it provides a means for obtaining data about a material's tensile or compressive strength without regard for the material's physical size or shape.
- *Engineering stress and strain* are calculated using the *original* cross-sectional area and gauge length of the specimen.
- A *ductile material*, such as mild steel, has four distinct behaviors as it is loaded. They are *elastic behavior*, *yielding*, *strain hardening*, and *necking*.
- A material is *linear elastic* if the stress is proportional to the strain within the elastic region. This behavior is described by *Hooke's law*,  $\sigma = E\epsilon$ , where the *modulus of elasticity*  $E$  is the slope of the line.
- Important points on the stress–strain diagram are the *proportional limit*, *elastic limit*, *yield stress*, *ultimate stress*, and *fracture stress*.

6



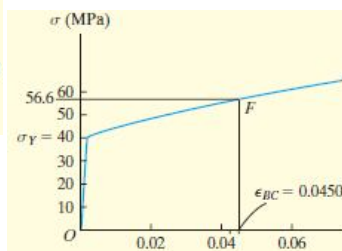
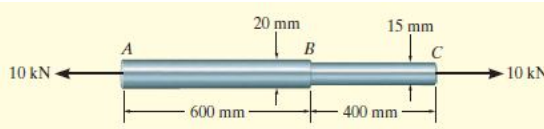
## Important Points

- The *ductility* of a material can be specified by the specimen's *percent elongation* or the *percent reduction in area*.
- If a material does not have a distinct yield point, a *yield strength* can be specified using a graphical procedure such as the *offset method*.
- *Brittle materials*, such as gray cast iron, have very little or no yielding and so they can fracture suddenly.
- *Strain hardening* is used to establish a higher yield point for a material. This is done by straining the material beyond the elastic limit, then releasing the load. The modulus of elasticity remains the same; however, the material's ductility *decreases*.
- *Strain energy* is energy stored in a material due to its deformation. This energy per unit volume is called *strain-energy density*. If it is measured up to the proportional limit, it is referred to as the *modulus of resilience*, and if it is measured up to the point of fracture, it is called the *modulus of toughness*. It can be determined from the area under the  $\sigma$ - $\epsilon$  diagram.

7

## EXAMPLE

An aluminum rod shown in Fig. 3-20*a* has a circular cross section and is subjected to an axial load of 10 kN. If a portion of the stress-strain diagram is shown in Fig. 3-20*b*, determine the approximate elongation of the rod when the load is applied. Take  $E_{al} = 70$  GPa.



$$\sigma_{AB} = \frac{P}{A} = \frac{10(10^3) \text{ N}}{\pi(0.01 \text{ m})^2} = 31.83 \text{ MPa}$$

$$\sigma_{BC} = \frac{P}{A} = \frac{10(10^3) \text{ N}}{\pi(0.0075 \text{ m})^2} = 56.59 \text{ MPa}$$

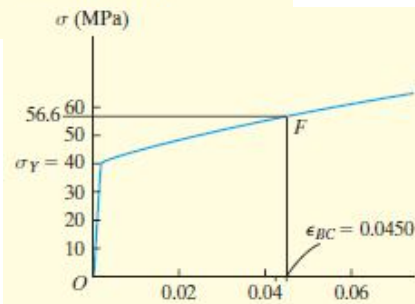
8

From the stress-strain diagram, the material in segment  $AB$  is strained *elastically* since  $\sigma_{AB} < \sigma_Y = 40$  MPa. Using Hooke's law,

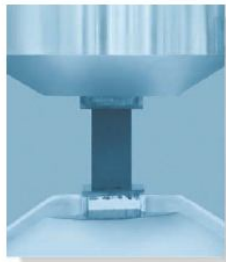
$$\epsilon_{AB} = \frac{\sigma_{AB}}{E_{al}} = \frac{31.83(10^6) \text{ Pa}}{70(10^9) \text{ Pa}} = 0.0004547 \text{ mm/mm}$$

The material within segment  $BC$  is strained plastically, since  $\sigma_{BC} > \sigma_Y = 40$  MPa. From the graph, for  $\sigma_{BC} = 56.59$  MPa,  $\epsilon_{BC} \approx 0.045$  mm/mm. The approximate elongation of the rod is therefore

$$\begin{aligned} \delta &= \Sigma \epsilon L = 0.0004547(600 \text{ mm}) + 0.0450(400 \text{ mm}) \\ &= 18.3 \text{ mm} \end{aligned}$$



### 3.6 Poisson's Ratio



When a deformable body is subjected to an axial tensile force, not only does it elongate but it also contracts laterally. For example, if a rubber band is stretched, it can be noted that both the thickness and width of the band are decreased. Likewise, a compressive force acting on a body causes it to contract in the direction of the force and yet its sides expand laterally.

Consider a bar having an original radius  $r$  and length  $L$  and subjected to the tensile force  $P$  in Fig. 3-21. This force elongates the bar by an amount  $\delta$ , and its radius contracts by an amount  $\delta'$ . Strains in the longitudinal or axial direction and in the lateral or radial direction are, respectively,

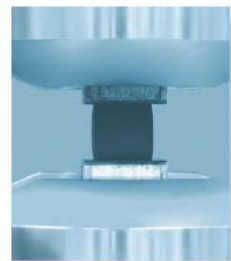
$$\epsilon_{\text{long}} = \frac{\delta}{L} \quad \text{and} \quad \epsilon_{\text{lat}} = \frac{\delta'}{r}$$

In the early 1800s, the French scientist S. D. Poisson realized that within the *elastic range* the *ratio* of these strains is a *constant*, since the deformations  $\delta$  and  $\delta'$  are proportional. This constant is referred to as *Poisson's ratio*,  $\nu$  (nu), and it has a numerical value that is unique for a particular material that is both *homogeneous* and *isotropic*. Stated mathematically it is

$$\nu = -\frac{\epsilon_{\text{lat}}}{\epsilon_{\text{long}}} \tag{3-9}$$

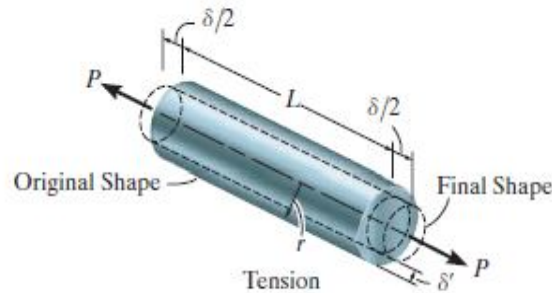
The negative sign is included here since *longitudinal elongation* (positive strain) causes *lateral contraction* (negative strain), and vice versa. Notice that these strains are caused only by the axial or longitudinal force  $P$ ; i.e., no force or stress acts in a lateral direction in order to strain the material in this direction.

Poisson's ratio is a *dimensionless* quantity, and for most nonporous solids it has a value that is generally between  $\frac{1}{4}$  and  $\frac{1}{2}$ . Typical values of  $\nu$  for common engineering materials are listed on the inside back cover.



When the rubber block is compressed (negative strain) its sides will expand (positive strain). The ratio of these strains remains constant.

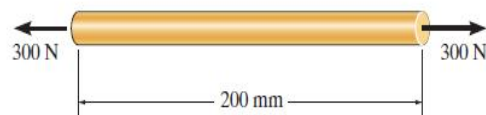
For an “ideal material” having no lateral deformation when it is stretched or compressed Poisson’s ratio will be 0. Furthermore, it will be shown in Sec. 10.6 that the *maximum* possible value for Poisson’s ratio is 0.5. Therefore  $0 \leq \nu \leq 0.5$ .



11

### EXAMPLE

**3-25.** The acrylic plastic rod is 200 mm long and 15 mm in diameter. If an axial load of 300 N is applied to it, determine the change in its length and the change in its diameter.  
 $E_p = 2.70 \text{ GPa}$ ,  $\nu_p = 0.4$ .



$$\sigma = \frac{P}{A} = \frac{300}{\frac{\pi}{4}(0.015)^2} = 1.678 \text{ MPa}$$

$$\epsilon_{\text{long}} = \frac{\sigma}{E} = \frac{1.678(10^6)}{2.70(10^9)} = 0.0006288$$

$$\delta = \epsilon_{\text{long}} L = 0.0006288 (200) = 0.126 \text{ mm}$$

$$\epsilon_{\text{lat}} = -\nu \epsilon_{\text{long}} = -0.4(0.0006288) = -0.0002515$$

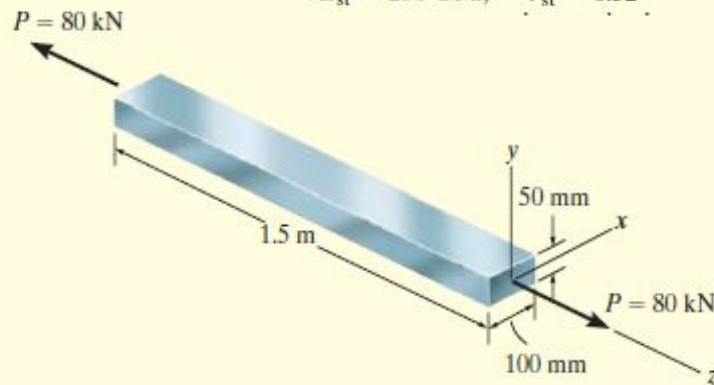
$$\Delta d = \epsilon_{\text{lat}} d = -0.0002515 (15) = -0.00377 \text{ mm}$$

12

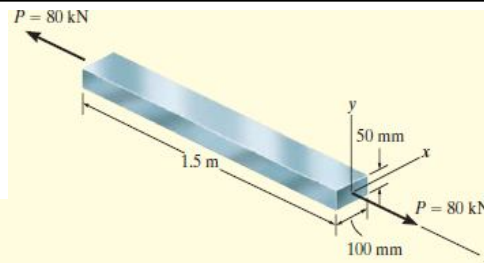
**EXAMPLE**

A bar made of A-36 steel has the dimensions shown in Fig. 3–22. If an axial force of  $P = 80 \text{ kN}$  is applied to the bar, determine the change in its length and the change in the dimensions of its cross section after applying the load. The material behaves elastically.

$$E_{st} = 200 \text{ GPa}, \quad \nu_{st} = 0.32$$



13



The normal stress in the bar is

$$\sigma_z = \frac{P}{A} = \frac{80(10^3) \text{ N}}{(0.1 \text{ m})(0.05 \text{ m})} = 16.0(10^6) \text{ Pa}$$

$$\epsilon_z = \frac{\sigma_z}{E_{st}} = \frac{16.0(10^6) \text{ Pa}}{200(10^9) \text{ Pa}} = 80(10^{-6}) \text{ mm/mm}$$

The axial elongation of the bar is therefore

$$\delta_z = \epsilon_z L_z = [80(10^{-6})](1.5 \text{ m}) = 120 \mu\text{m}$$

$$\epsilon_x = \epsilon_y = -\nu_{st} \epsilon_z = -0.32[80(10^{-6})] = -25.6 \mu\text{m/m}$$

Thus the changes in the dimensions of the cross section are

$$\delta_x = \epsilon_x L_x = -[25.6(10^{-6})](0.1 \text{ m}) = -2.56 \mu\text{m}$$

$$\delta_y = \epsilon_y L_y = -[25.6(10^{-6})](0.05 \text{ m}) = -1.28 \mu\text{m}$$

14



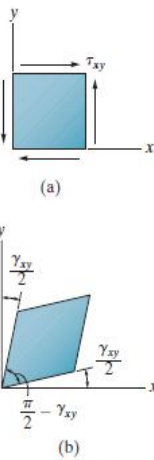


Fig. 3-23

### 3.7 The Shear Stress–Strain Diagram

In Sec. 1.5 it was shown that when a small element of material is subjected to *pure shear*, equilibrium requires that equal shear stresses must be developed on four faces of the element. These stresses  $\tau_{xy}$  must be directed toward or away from diagonally opposite corners of the element, as shown in Fig. 3-23a. Furthermore, if the material is *homogeneous* and *isotropic*, then this shear stress will distort the element uniformly, Fig. 3-23b. As mentioned in Sec. 2.2, the shear strain  $\gamma_{xy}$  measures the angular distortion of the element relative to the sides originally along the  $x$  and  $y$  axes.

The behavior of a material subjected to pure shear can be studied in a laboratory using specimens in the shape of thin tubes and subjecting them to a torsional loading. If measurements are made of the applied torque and the resulting angle of twist, then by the methods to be explained in Chapter 5, the data can be used to determine the shear stress and shear strain, and a shear stress–strain diagram plotted. An example of such a diagram for a ductile material is shown in Fig. 3-24. Like the tension test, this material when subjected to shear will exhibit linear-elastic behavior and it will have a defined *proportional limit*  $\tau_{pl}$ . Also, strain hardening will occur until an *ultimate shear stress*  $\tau_u$  is reached. And finally, the material will begin to lose its shear strength until it reaches a point where it fractures,  $\tau_f$ .

15

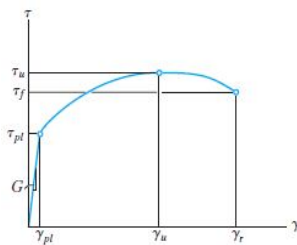


Fig. 3-24

For most engineering materials, like the one just described, the elastic behavior is *linear*, and so Hooke’s law for shear can be written as

$$\tau = G\gamma \tag{3-10}$$

Here  $G$  is called the *shear modulus of elasticity* or the *modulus of rigidity*. Its value represents the slope of the line on the  $\tau$ – $\gamma$  diagram, that is,  $G = \tau_{pl}/\gamma_{pl}$ . Typical values for common engineering materials are listed on the inside back cover. Notice that the units of measurement for  $G$  will be the *same* as those for  $\tau$  (Pa or psi), since  $\gamma$  is measured in radians, a dimensionless quantity.

It will be shown in Sec. 10.6 that the three material constants,  $E$ ,  $\nu$ , and  $G$  are actually *related* by the equation

$$G = \frac{E}{2(1 + \nu)} \tag{3-11}$$

Provided  $E$  and  $G$  are known, the value of  $\nu$  can then be determined from this equation rather than through experimental measurement. For example, in the case of A-36 steel,  $E_{st} = 29(10^3)$  ksi and  $G_{st} = 11.0(10^3)$  ksi, so that, from Eq. 3-11,  $\nu_{st} = 0.32$ .

16



**EXAMPLE**

An aluminum specimen shown in Fig. 3–26 has a diameter of  $d_0 = 25$  mm and a gauge length of  $L_0 = 250$  mm. If a force of 165 kN elongates the gauge length 1.20 mm, determine the modulus of elasticity. Also, determine by how much the force causes the diameter of the specimen to contract. Take  $G_{al} = 26$  GPa and  $\sigma_Y = 440$  MPa.

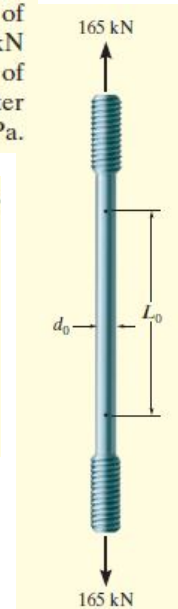
**Modulus of Elasticity.** The average normal stress in the specimen is

$$\sigma = \frac{P}{A} = \frac{165(10^3) \text{ N}}{(\pi/4)(0.025 \text{ m})^2} = 336.1 \text{ MPa}$$

and the average normal strain is

$$\epsilon = \frac{\delta}{L} = \frac{1.20 \text{ mm}}{250 \text{ mm}} = 0.00480 \text{ mm/mm}$$

$$E_{al} = \frac{\sigma}{\epsilon} = \frac{336.1(10^6) \text{ Pa}}{0.00480} = 70.0 \text{ GPa}$$



17

$$G = \frac{E}{2(1 + \nu)}$$

$$26 \text{ GPa} = \frac{70.0 \text{ GPa}}{2(1 + \nu)}$$

$$\nu = 0.347$$

Since  $\epsilon_{\text{long}} = 0.00480$  mm/mm, then by

$$\nu = -\frac{\epsilon_{\text{lat}}}{\epsilon_{\text{long}}}$$

$$0.347 = -\frac{\epsilon_{\text{lat}}}{0.00480 \text{ mm/mm}}$$

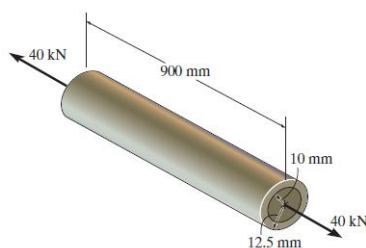
$$\epsilon_{\text{lat}} = -0.00166 \text{ mm/mm}$$

The contraction of the diameter is therefore

$$\begin{aligned} \delta' &= (0.00166)(25 \text{ mm}) \\ &= 0.0416 \text{ mm} \end{aligned}$$

18

**3-26.** The thin-walled tube is subjected to an axial force of 40 kN. If the tube elongates 3 mm and its circumference decreases 0.09 mm, determine the modulus of elasticity, Poisson's ratio, and the shear modulus of the tube's material. The material behaves elastically.



**Normal Stress and Strain:**

$$\sigma = \frac{P}{A} = \frac{40(10^3)}{\pi(0.0125^2 - 0.01^2)} = 226.35 \text{ MPa}$$

$$\epsilon_a = \frac{\delta}{L} = \frac{3}{900} = 3.3333 (10^{-3}) \text{ mm/mm}$$

Applying Hooke's law,

$$\sigma = E\epsilon_a; \quad 226.35(10^6) = E[3.3333(10^{-3})]$$

$$E = 67.91(10^6) \text{ Pa} = 67.9 \text{ GPa}$$

$$G = \frac{E}{2(1 + \nu)} = \frac{67.91(10^9)}{2(1 + 0.3438)} = 25.27(10^9) \text{ Pa} = 25.3 \text{ GI}$$

**Poisson's Ratio:** The circumference of the loaded tube is  $2\pi(12.5) - 0.09 = 78.4498$  mm. Thus, the outer radius of the tube is

$$r = \frac{78.4498}{2\pi} = 12.4857 \text{ mm}$$

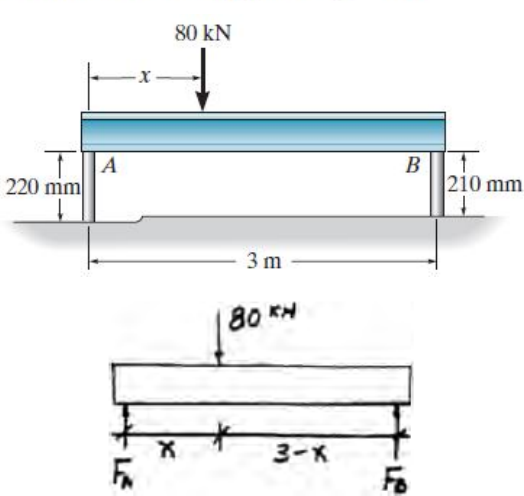
lateral strain is

$$\epsilon_{\text{lat}} = \frac{r - r_0}{r_0} = \frac{12.4857 - 12.5}{12.5} = -1.1459(10^{-3}) \text{ mm/mm}$$

$$\nu = -\frac{\epsilon_{\text{lat}}}{\epsilon_a} = -\left[\frac{-1.1459(10^{-3})}{3.3333(10^{-3})}\right] = 0.3438 = 0.344$$

19

**3-37.** The rigid beam rests in the horizontal position on two 2014-T6 aluminum cylinders having the *unloaded* lengths shown. If each cylinder has a diameter of 30 mm. determine the placement  $x$  of the applied 80-kN load so that the beam remains horizontal. What is the new diameter of cylinder A after the load is applied?  $\nu_{\text{al}} = 0.35$ .



20

$$\zeta + \sum M_A = 0; \quad F_B(3) - 80(x) = 0; \quad F_B = \frac{80x}{3}$$

$$\zeta + \sum M_B = 0; \quad -F_A(3) + 80(3-x) = 0; \quad F_A = \frac{80(3-x)}{3}$$

Since the beam is held horizontally,  $\delta_A = \delta_B$

$$\sigma = \frac{P}{A}; \quad \varepsilon = \frac{\sigma}{E} = \frac{P}{AE}$$

$$\delta = \varepsilon L = \left(\frac{P}{AE}\right)L = \frac{PL}{AE}$$

$$\delta_A = \delta_B; \quad \frac{80(3-x)(220)}{AE} = \frac{80x(210)}{AE}$$

$$80(3-x)(220) = 80x(210)$$

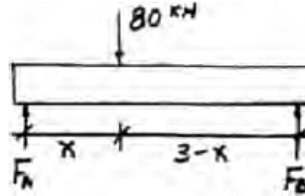
$$x = 1.53 \text{ m}$$

From Eq. (2),

$$F_A = 39.07 \text{ kN}$$

$$\sigma_A = \frac{F_A}{A} = \frac{39.07(10^3)}{\frac{\pi}{4}(0.03^2)} = 55.27 \text{ MPa}$$

$$\varepsilon_{\text{long}} = \frac{\sigma_A}{E} = \frac{55.27(10^6)}{73.1(10^9)} = -0.000756$$



$$\varepsilon_{\text{lat}} = -\nu \varepsilon_{\text{long}} = -0.35(-0.000756) = 0.0002646$$

$$d'_A = d_A + d \varepsilon_{\text{lat}} = 30 + 30(0.0002646) = 30.008 \text{ mm}$$

21

\*3-44. The A-36 steel wire  $AB$  has a cross-sectional area of  $10 \text{ mm}^2$  and is unstretched when  $\theta = 45.0^\circ$ . Determine the applied load  $P$  needed to cause  $\theta = 44.9^\circ$ .

$$\frac{L'_{AB}}{\sin 90.2^\circ} = \frac{400}{\sin 44.9^\circ}$$

$$L'_{AB} = 566.67 \text{ mm}$$

$$L_{AB} = \frac{400}{\sin 45^\circ} = 565.69$$

$$\varepsilon = \frac{L'_{AB} - L_{AB}}{L_{AB}} = \frac{566.67 - 565.69}{565.69} = 0.001744$$

$$\sigma = E\varepsilon = 200(10^9)(0.001744) = 348.76 \text{ MPa}$$

$$\zeta + \sum M_A = 0$$

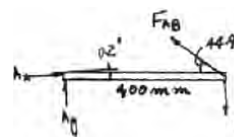
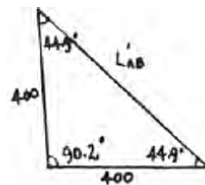
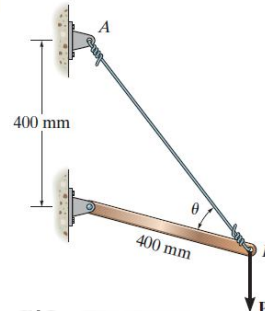
$$P(400 \cos 0.2^\circ) - F_{AB} \sin 44.9^\circ (400) = 0$$

However,

$$F_{AB} = \sigma A = 348.76(10^6)(10)(10^{-6}) = 3.488 \text{ kN}$$

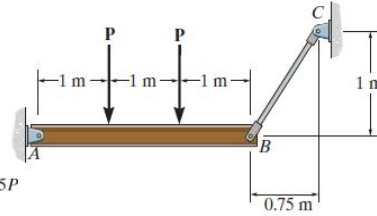
From Eq. (1),

$$P = 2.46 \text{ kN}$$



22

3-27. When the two forces are placed on the beam, the diameter of the A-36 steel rod  $BC$  decreases from 40 mm to 39.99 mm. Determine the magnitude of each force  $P$ .



$$\zeta + \sum M_A = 0; \quad F_{BC} \left( \frac{4}{5} \right) (3) - P(2) - P(1) = 0 \quad F_{BC} = 1.25P$$

**Normal Stress and Strain:** The lateral strain of rod  $BC$  is

$$\epsilon_{\text{lat}} = \frac{d - d_0}{d_0} = \frac{39.99 - 40}{40} = -0.25(10^{-3}) \text{ mm/mm}$$

$$\epsilon_{\text{lat}} = -\nu \epsilon_a; \quad -0.25(10^{-3}) = -(0.32) \epsilon_a$$

$$\epsilon_a = 0.78125(10^{-3}) \text{ mm/mm}$$

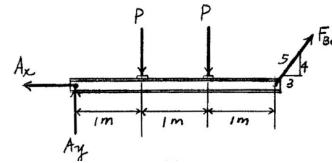
Assuming that Hooke's Law applies,

$$\sigma_{BC} = E \epsilon_a; \quad \sigma_{BC} = 200(10^9)(0.78125)(10^{-3}) = 156.25 \text{ MPa}$$

Since  $\sigma < \sigma_Y$ , the assumption is correct.

$$\sigma_{BC} = \frac{F_{BC}}{A_{BC}}; \quad 156.25(10^6) = \frac{1.25P}{\frac{\pi}{4}(0.04^2)}$$

$$P = 157.08(10^3) \text{ N} = 157 \text{ kN}$$



A

23